

Supplemental Post-Tensioning for Concrete Dome Repairs

Travis Marman, Cody Mitchell, Josh Brewster

Vector Construction, Inc.
Fargo, ND

Jared Brewe, Neal Anderson

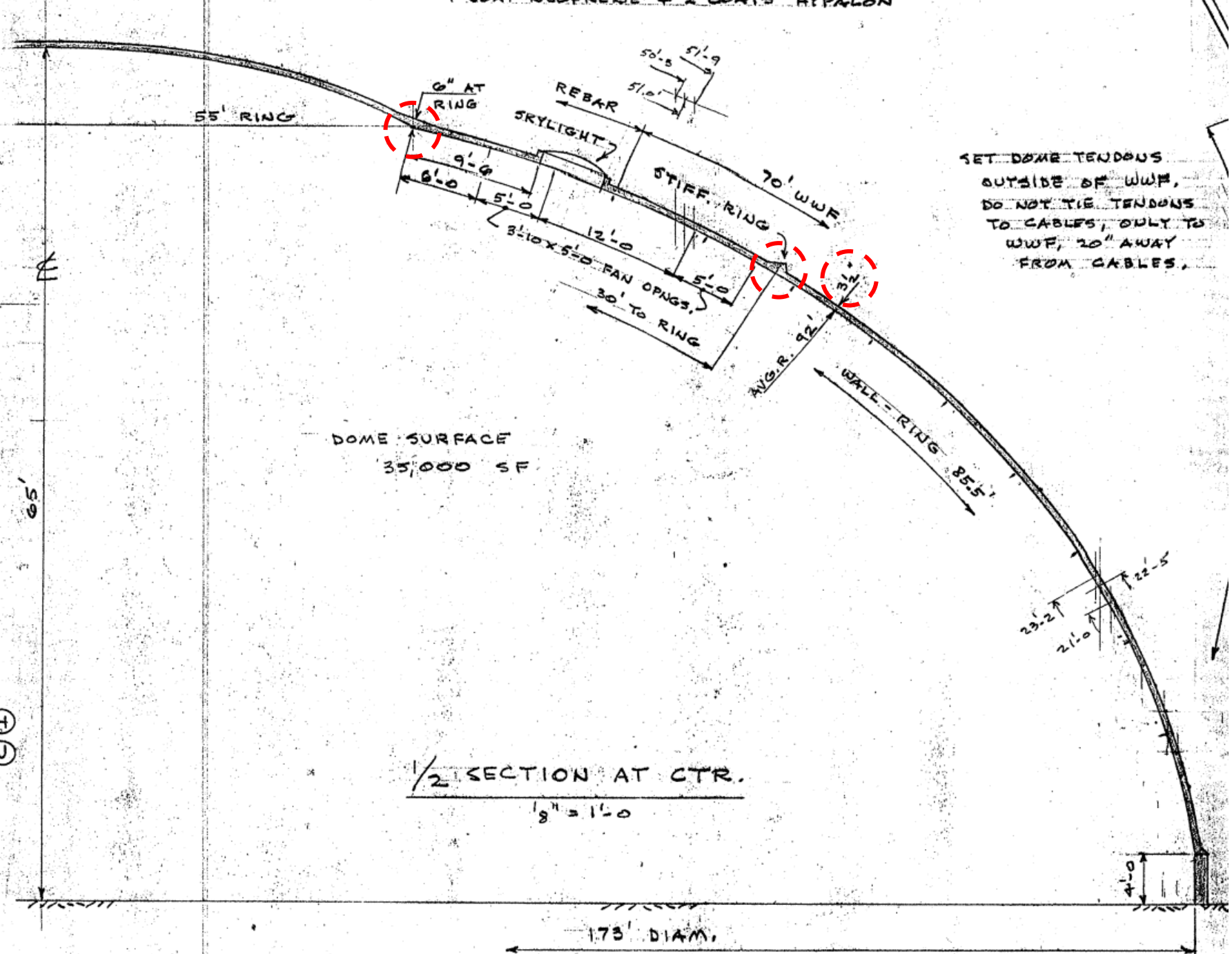
Simpson Gumpertz & Heger
Chicago, IL

Matt Miltenberger, Andrew Broecker

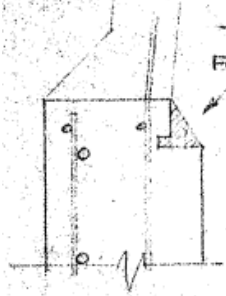
Vector Corrosion Services, Inc.
Tampa, FL







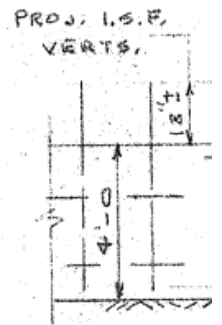
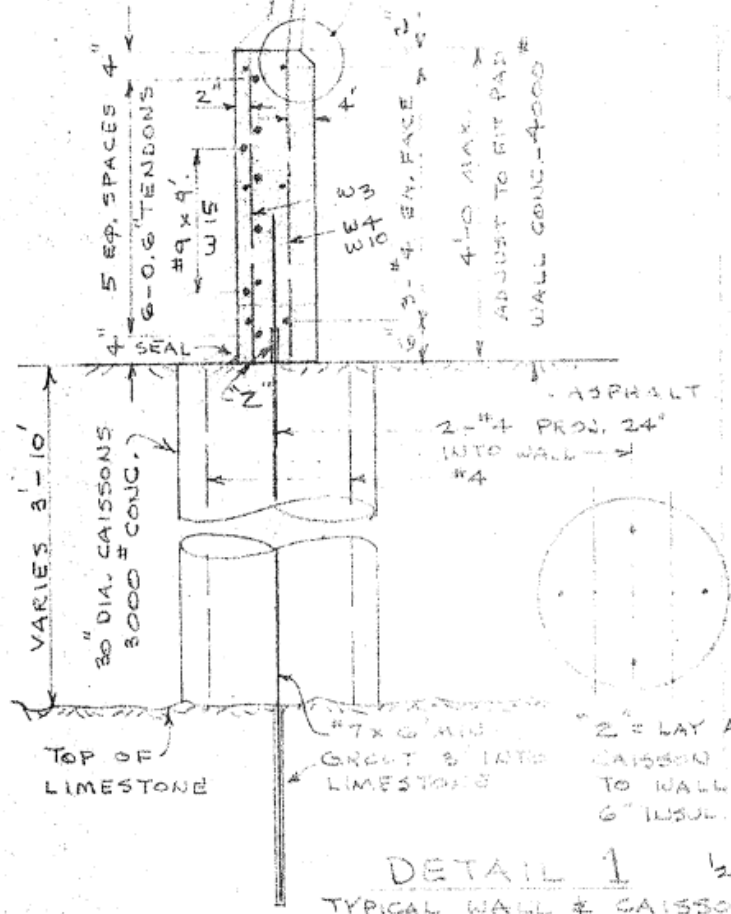
SET DOME TENDONS
OUTSIDE OF WWF,
DO NOT TIE TENDONS
TO CABLES, ONLY TO
WWF, 20" AWAY
FROM CABLES.



TROWEL IN AFTER FORM REMOVED

DOME SHELL - 3 H. 3

WS = #4 x 4' @ 23" +
 WT = #4 x 5' @ 23" +
 (EXCEPT #5 AT 3'-10" FROM EACH CORNER, W10)

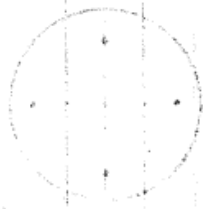


VARIES 3'-10"

VARIES 3'-10"
 30" DIA. CAISSONS
 8000 # CONG.

TOP OF LIMESTONE

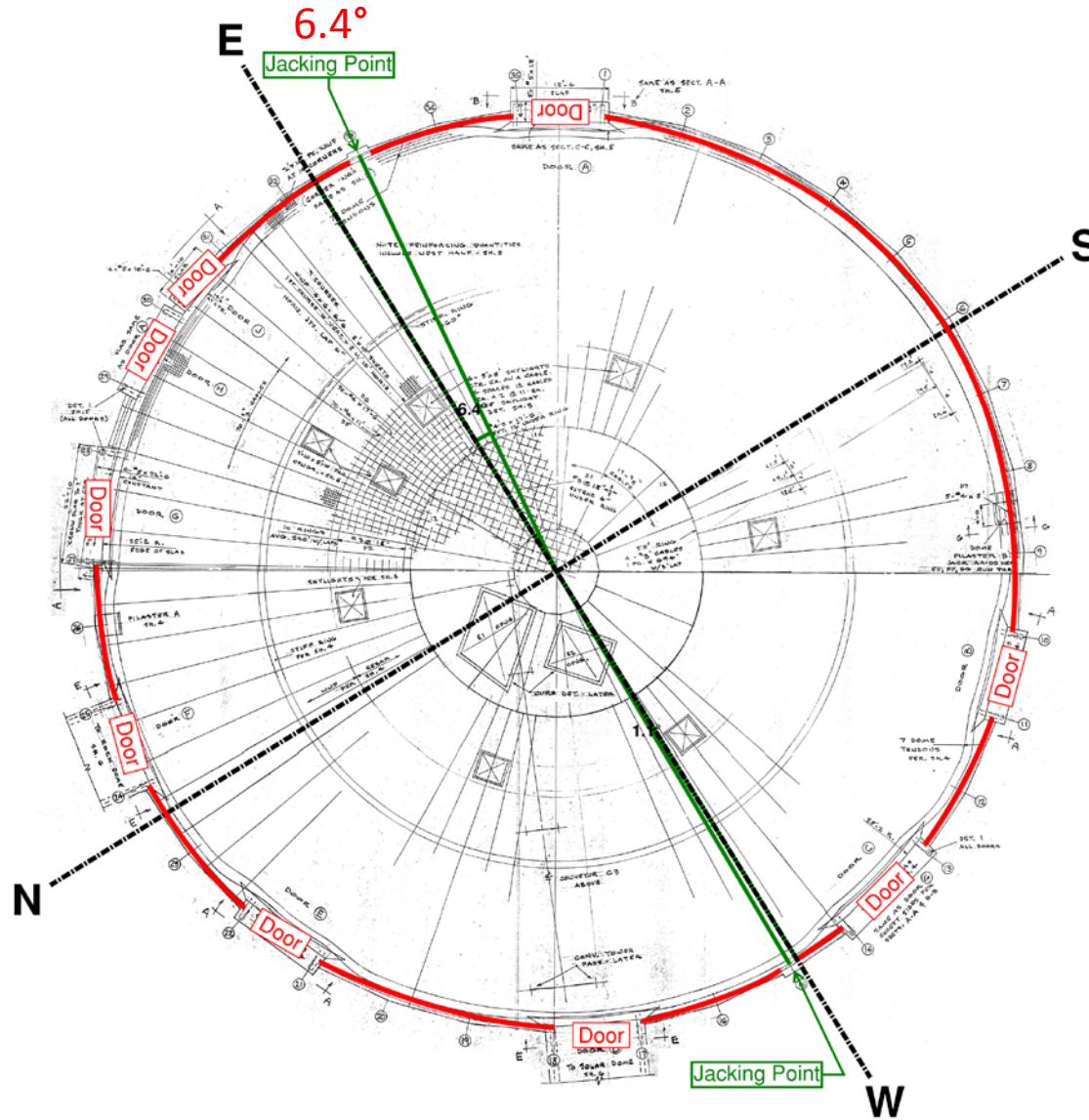
ASPHALT
 2-#4 PROJ. 24" INTO WALL - 1
 #4

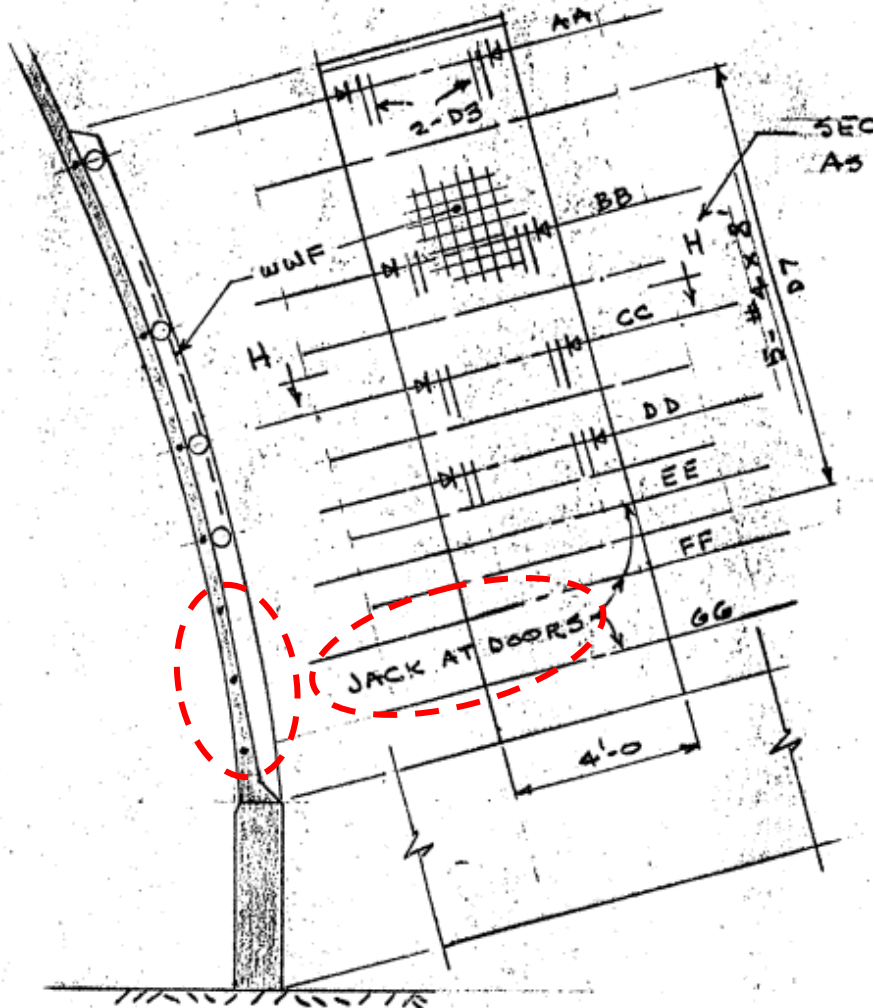


2 LAY ASPHALT FELT ON CAISSON TO PREVENT BOND TO WALL CONCRETE. SET 6" INSUL. SLEEVES ON #4'S

DETAIL 1 1/2" = 1'-0"
 TYPICAL WALL & CAISSON

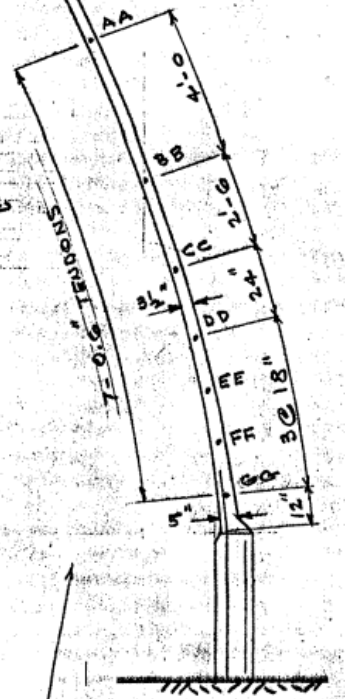
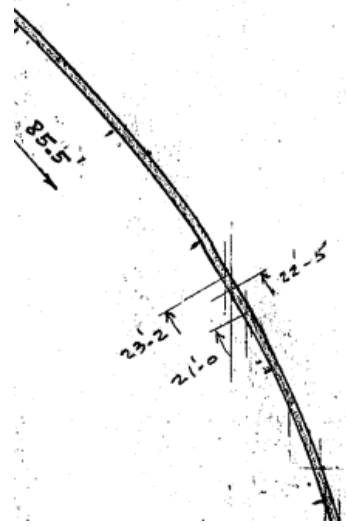
Location of Door Jambs



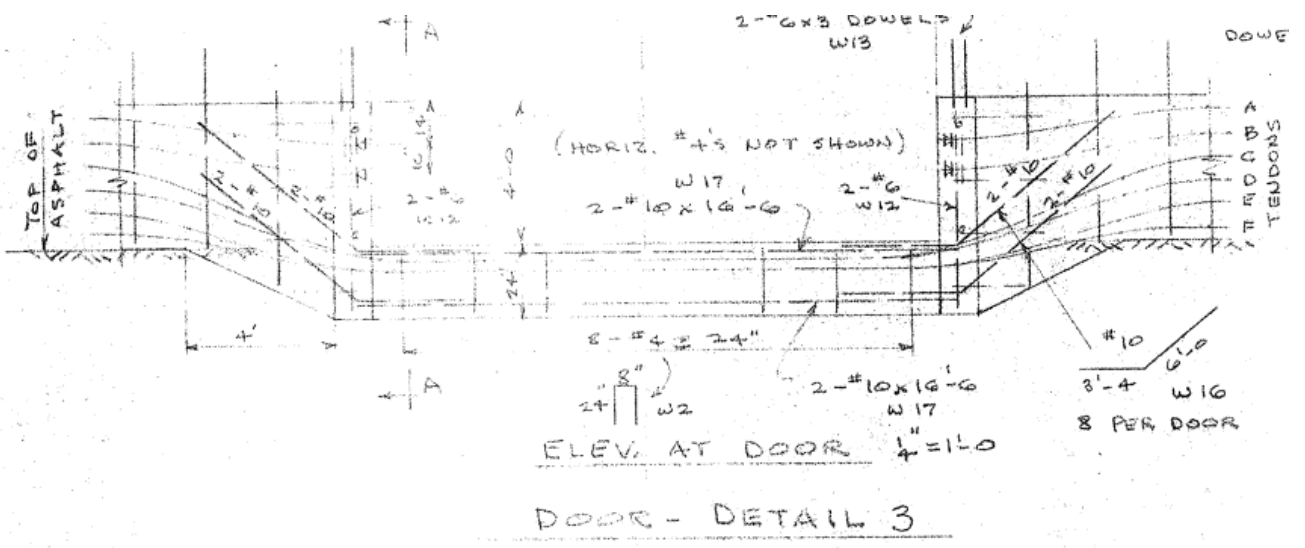
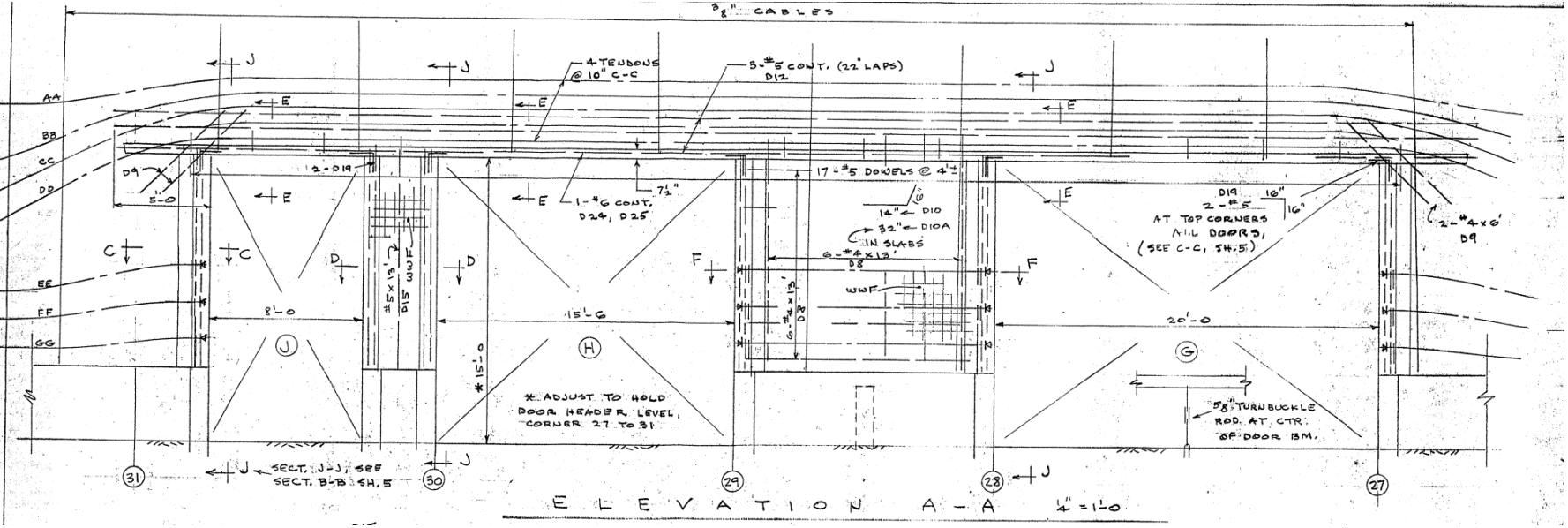


SECT. G-G
 1/4" = 1'-0"
PILASTERS A & B

SET DOME TENDONS
 OUTSIDE OF WWF,
 DO NOT TIE TENDONS
 TO CABLES, ONLY TO
 WWF, 20" AWAY
 FROM CABLES.



LOWER DOME
DETAILS
 1/4" = 1'-0"
 (WWF & CABLES
 NOT SHOWN)



DOOR - DETAIL 3













Situation

- Structure ~ 30 yr old, Owner wants 30 more yr
- 24/7 operation
- Aggressive environment
- Concerns over leaking, cracking, and deflection
- Owner weighing options to rehab or replace
- Prior evaluation
 - Sounding - identified “wide-spread delamination”
 - Corrosion potentials – identified active corrosion
 - No mention of PT system status
 - Recommended replacement
 - Installed netting inside, external foam, and waterproofing





Investigation Phase

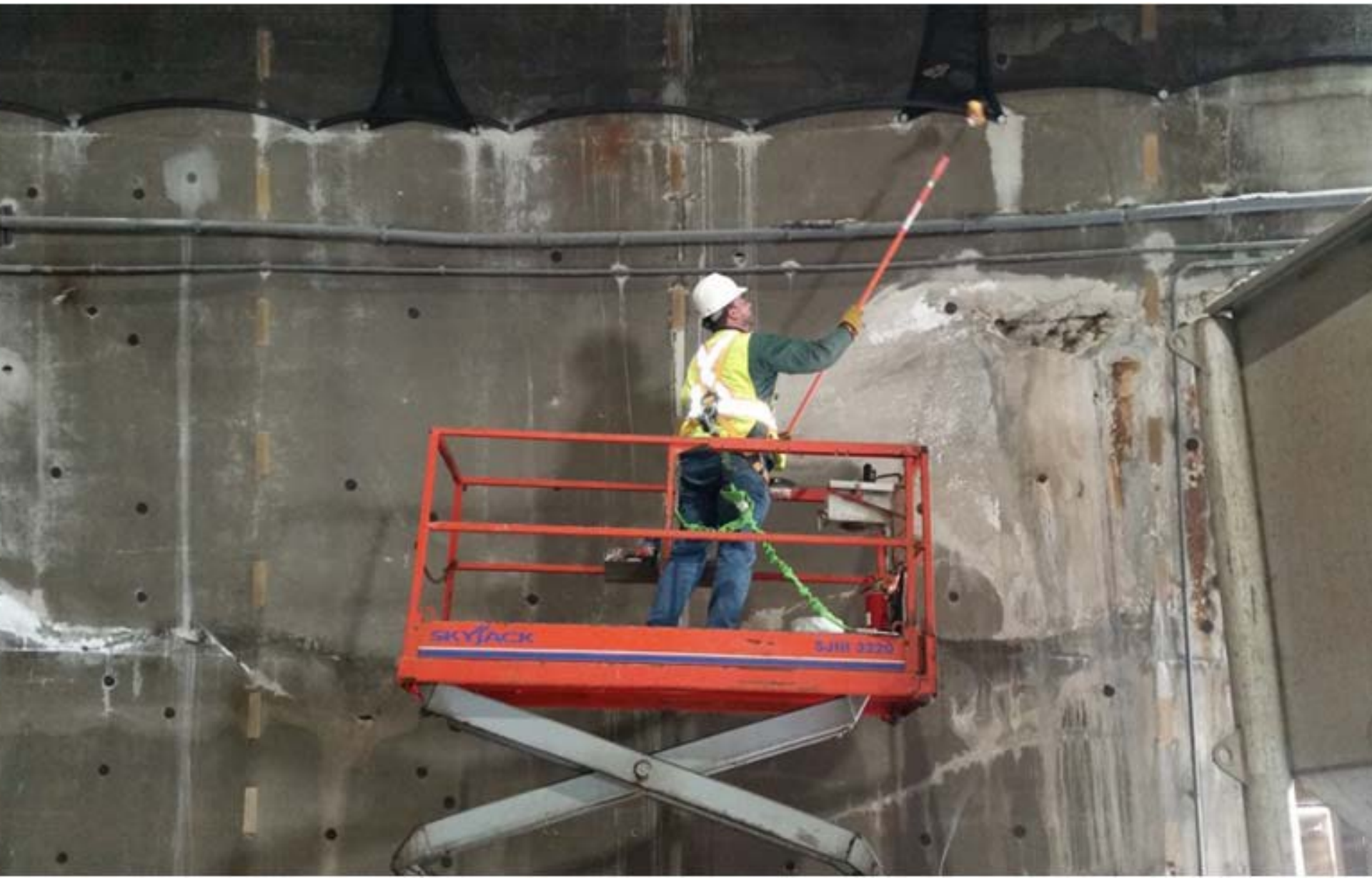
- Document Review
- Interviews with maintenance personnel
- Visual assessment
- Corrosion potential survey
- Chloride, carbonation, strength sampling
- PT evaluation
- LIDAR shape survey

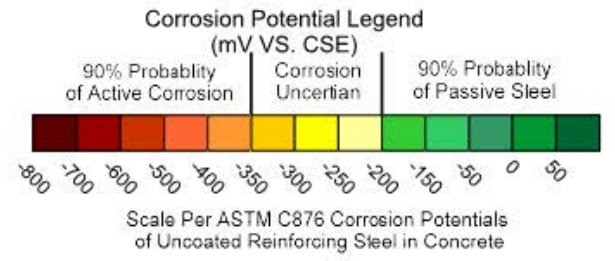
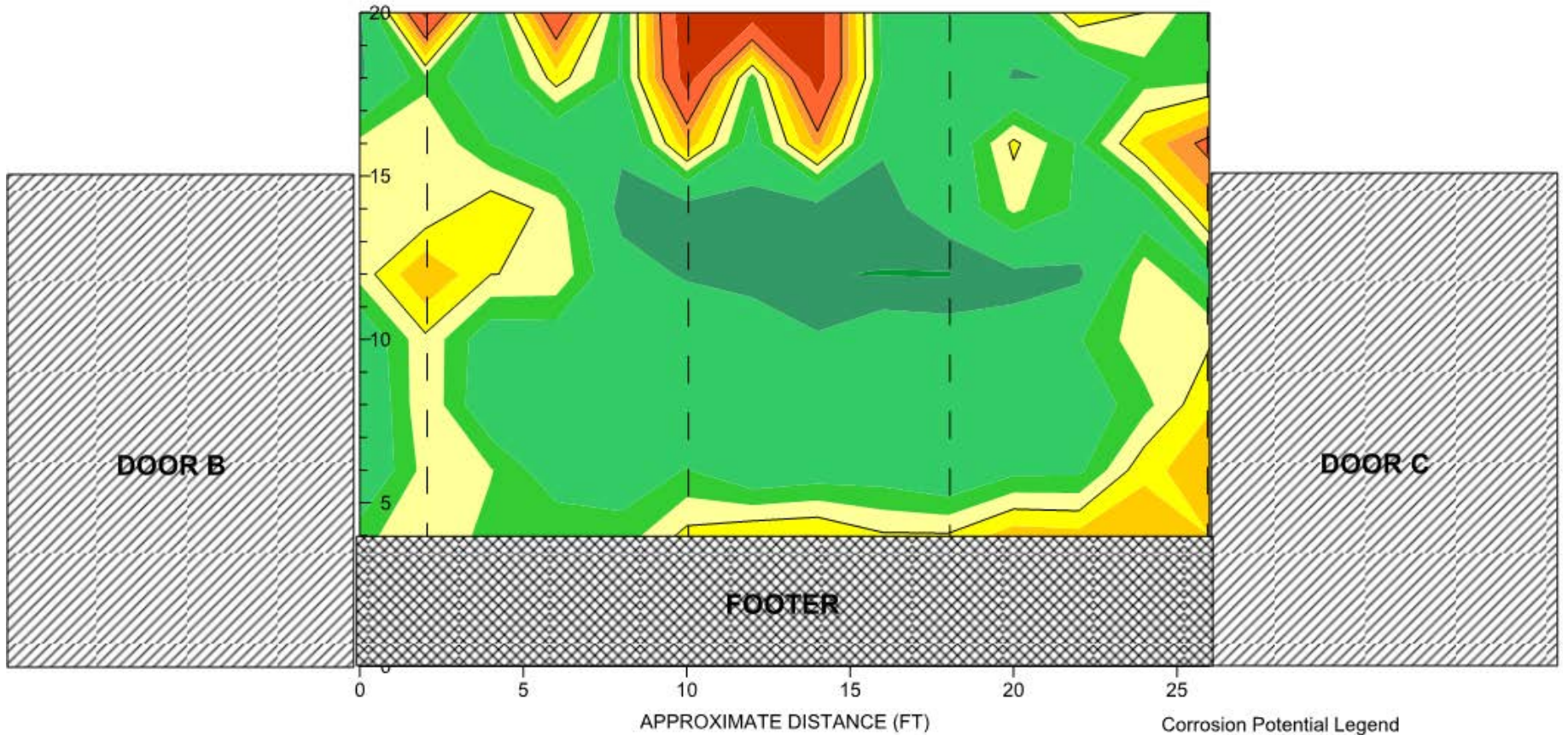


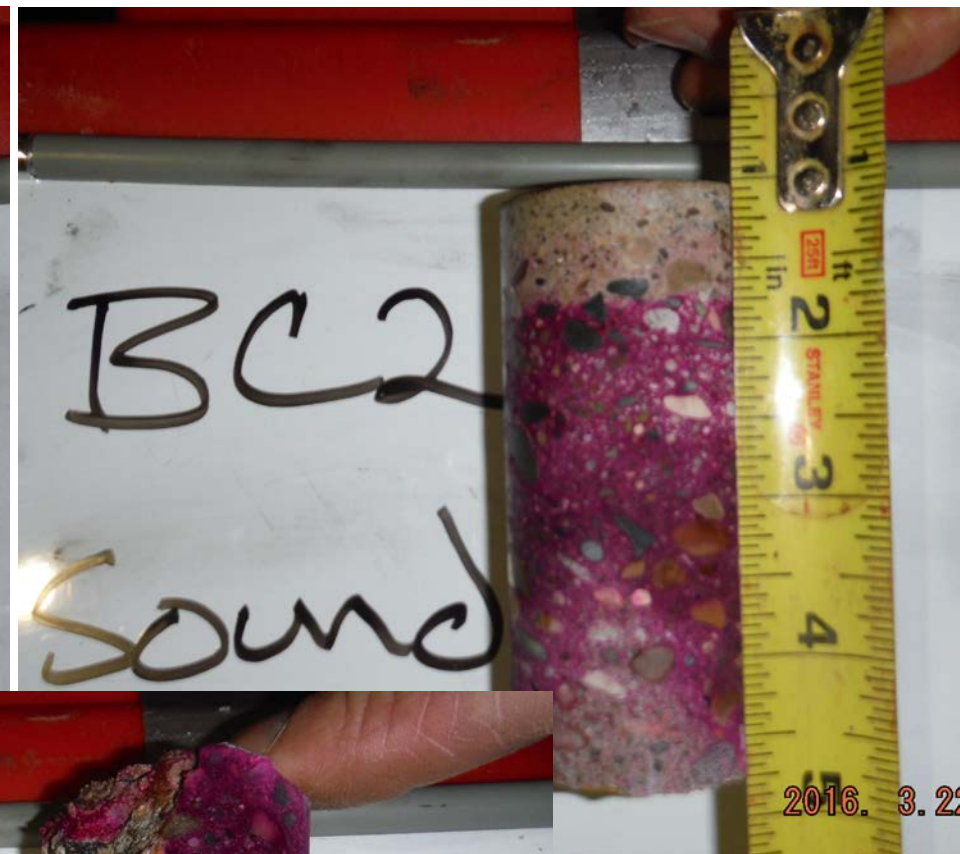


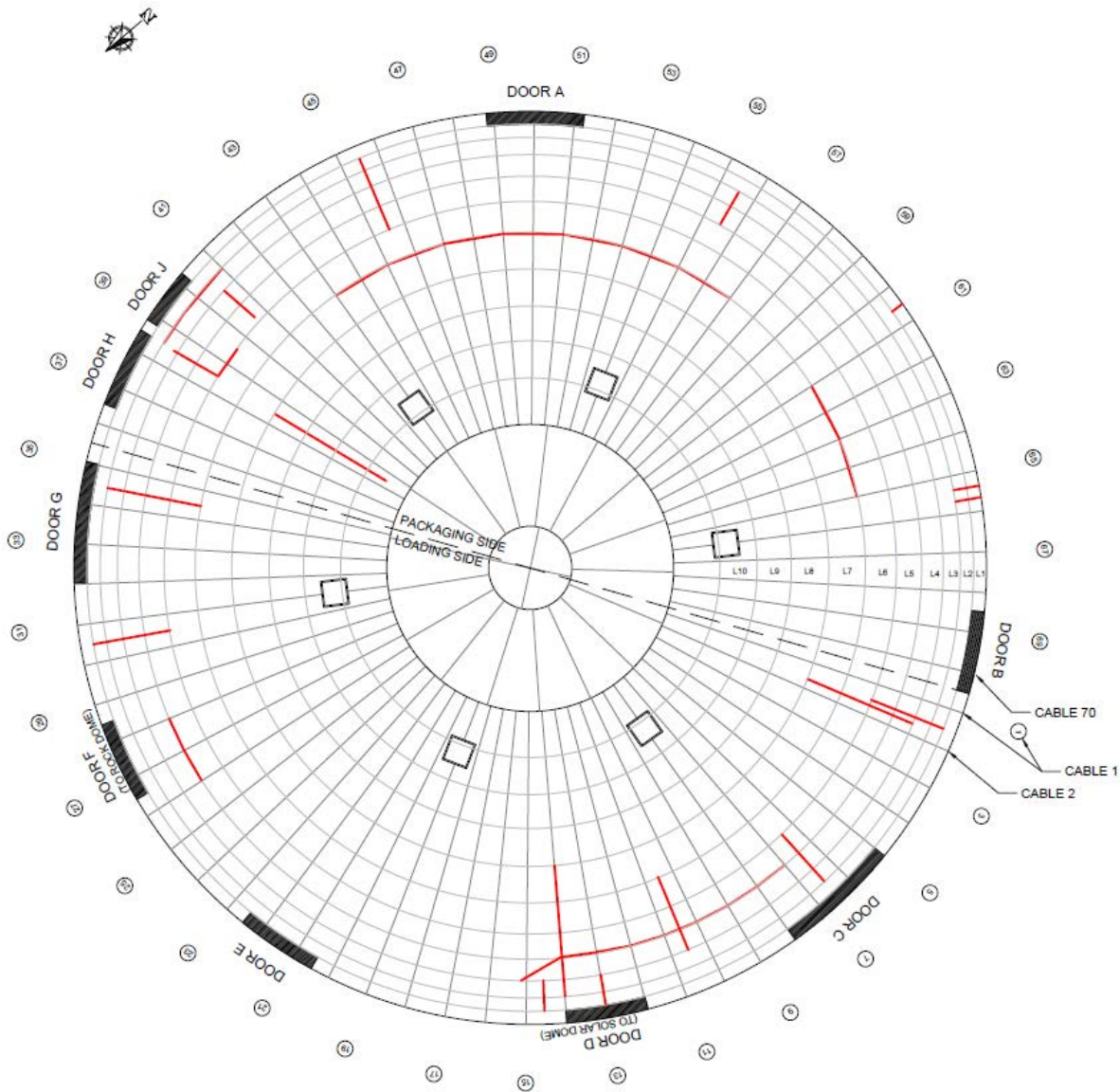








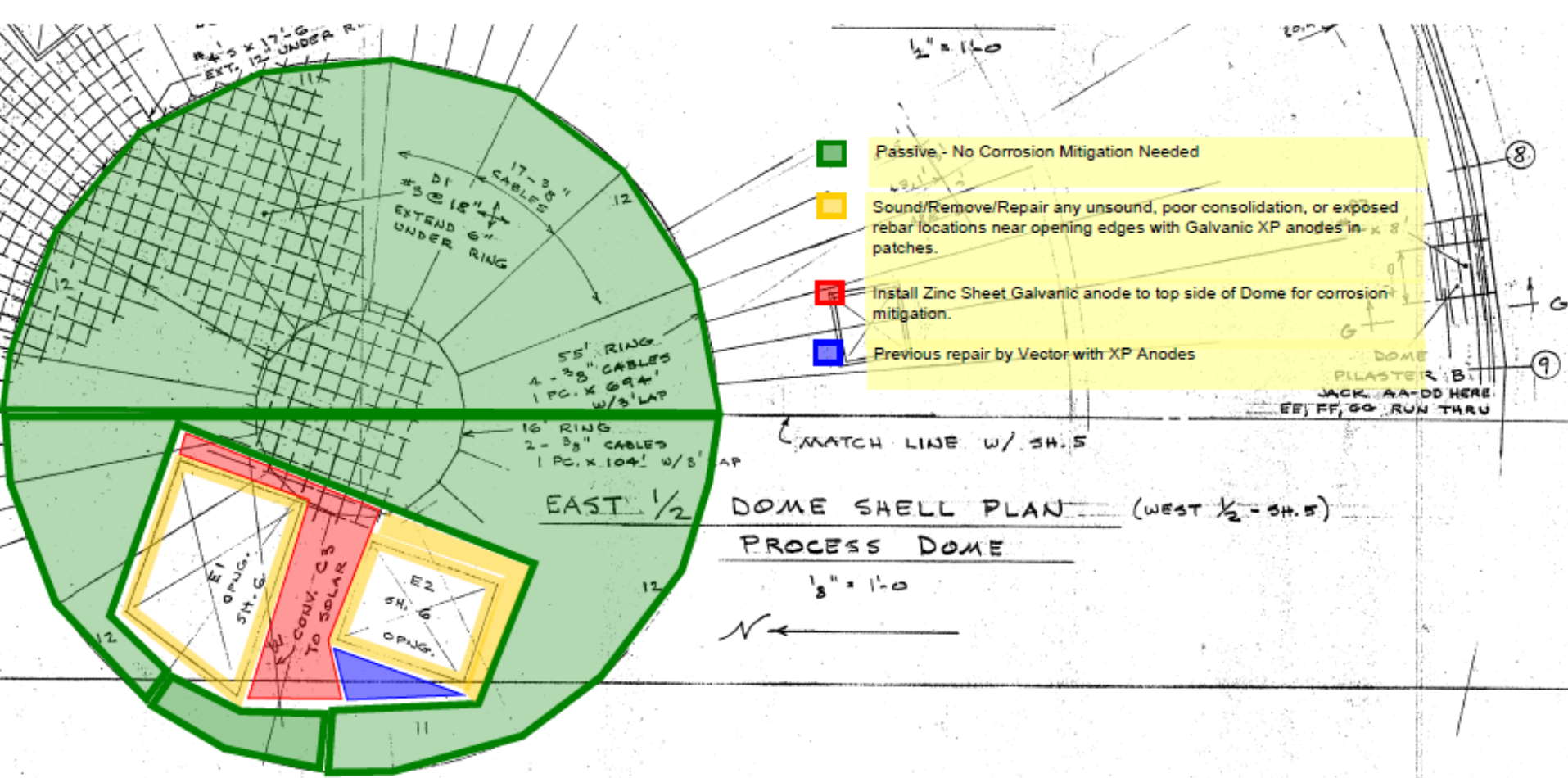




PLAN VIEW




2016. 3. 23 11:13




Notes:

- The majority of corrosion potentials taken at the top of the dome showed very passive conditions at the reinforcing steel. There were a select few localized active areas of corrosion indicated in the area between the two openings. This area is exposed from above and below with no waterproofing on top and is exposed to the humidity from outside the top penthouse. This entire area will be treated with a corrosion mitigation system to prevent against future corrosion.

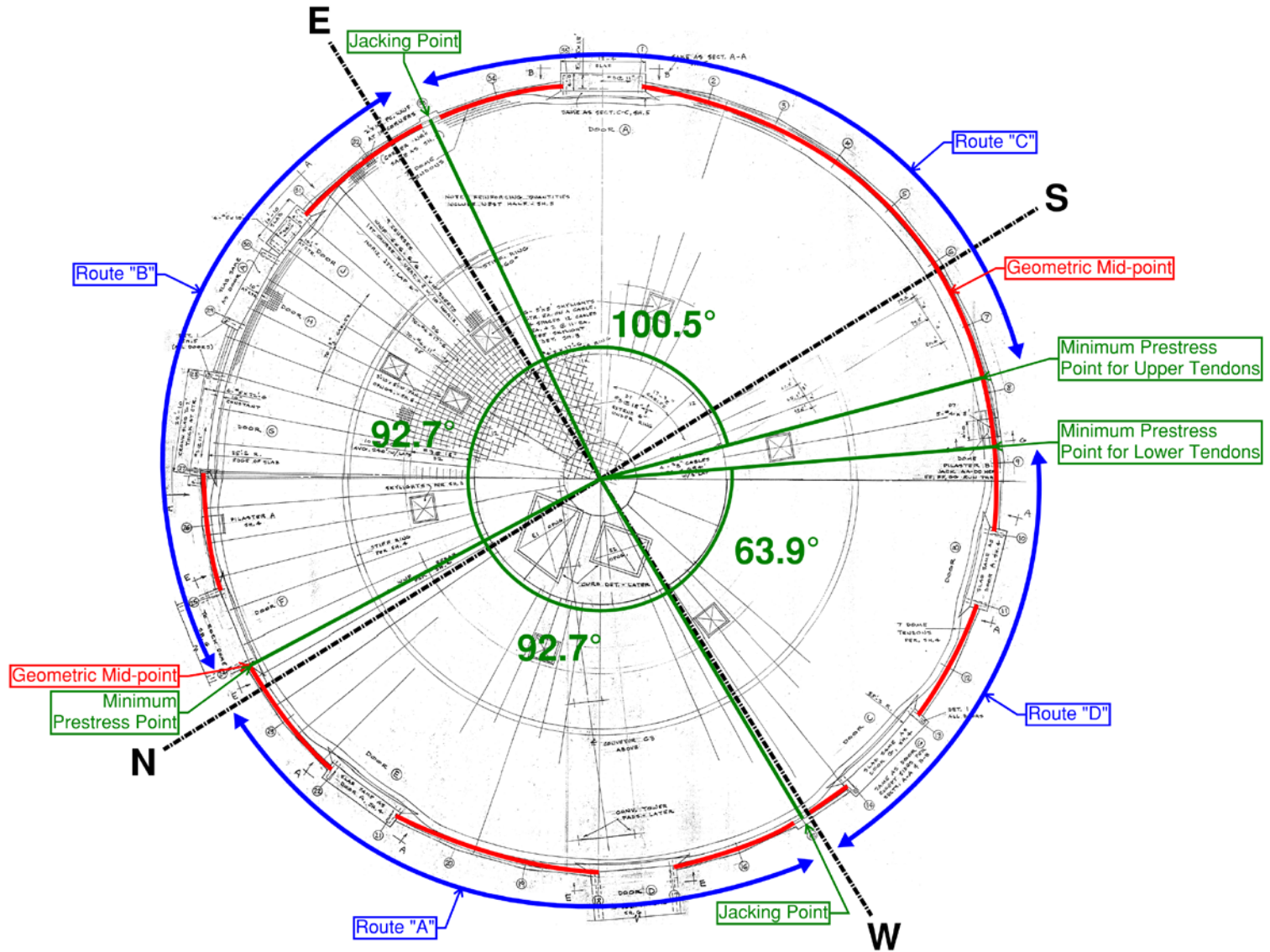
Findings

- Widespread shadowing behind reinforcing
 - Reinforcement was severed at leaking cracks
 - Corrosion damage around skylights
 - No prestress in any tendon tested
 - PT anchors exposed, corroding
 - Poor consolidation behind some PT anchors
 - Carbonation & chloride progressed along cracks
 - Concrete strength >7000 psi where consolidated
- 

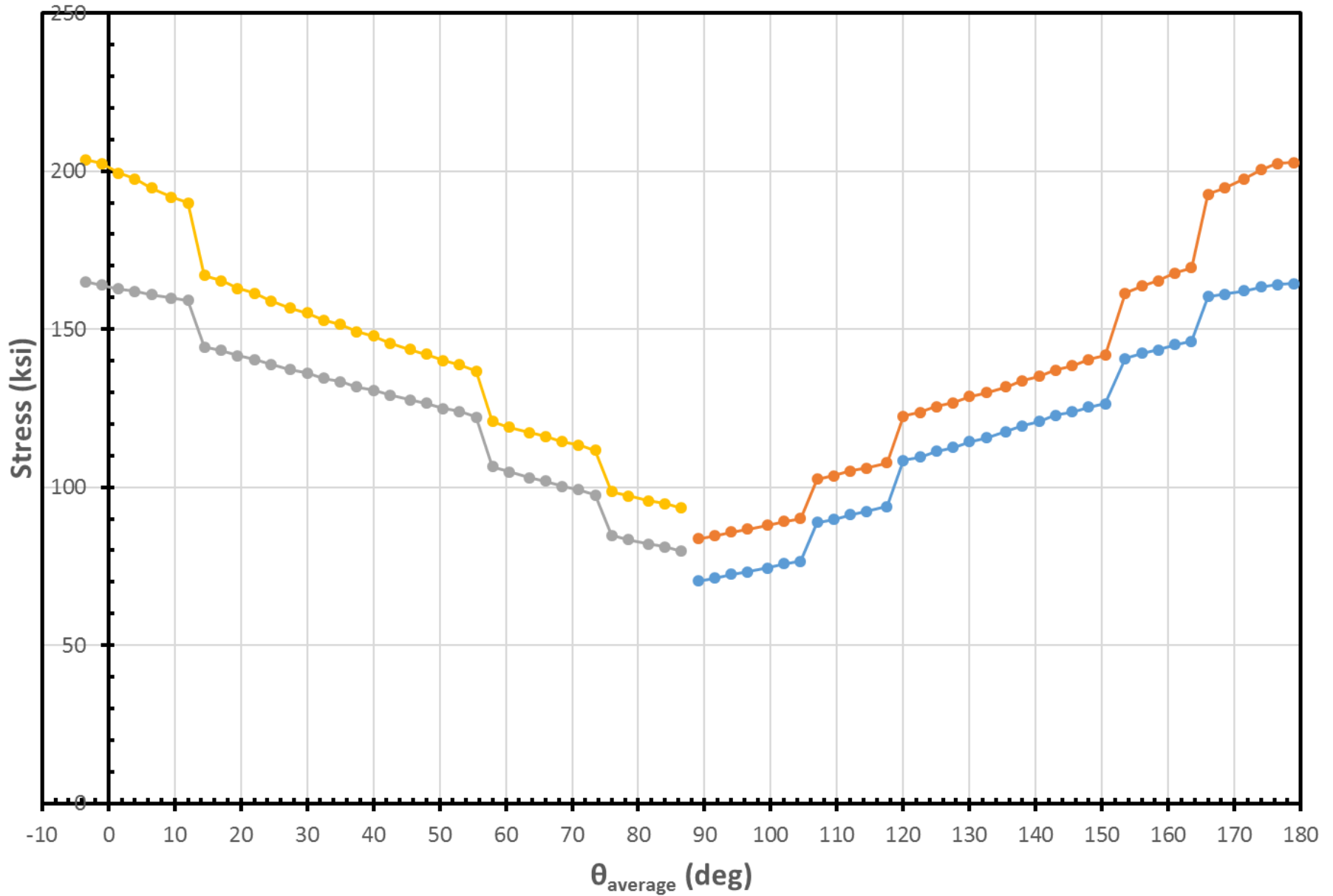
Rehabilitation Strategy

- Structural analysis required (FEM analysis)
 - Remove conveyor support loads
 - Infill skylights
 - Restraining PT between doors
 - Cast new door jambs and eyebrows
 - External PT above eyebrows
 - GFRP strengthening at top stiffening ring
 - Inject/seal cracks with epoxy
 - Corrosion mitigation at cracks and as needed
- 

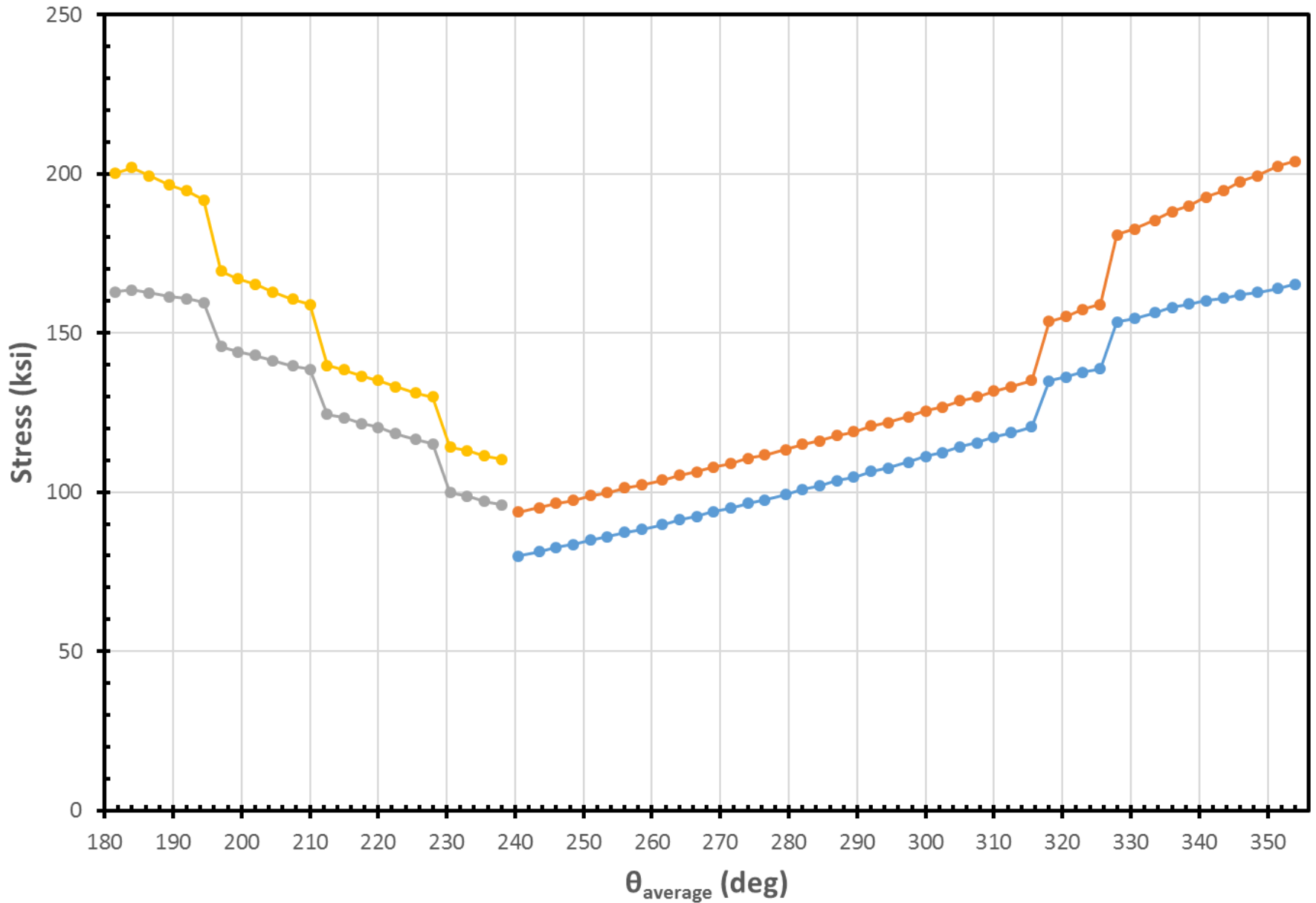
Schematic Jacking Stress Location Plan



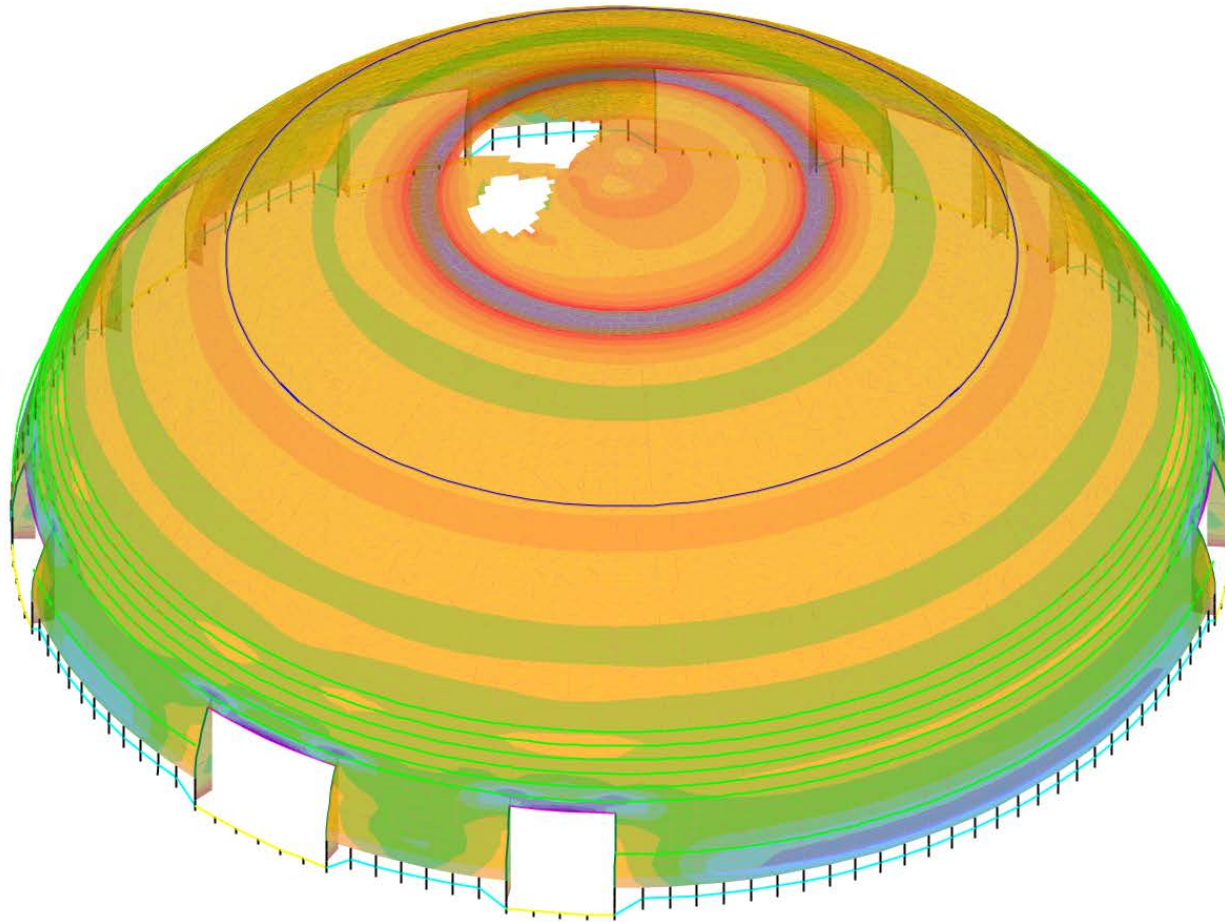
Initial and Long-Term Effective Tendon Stress (A-B)



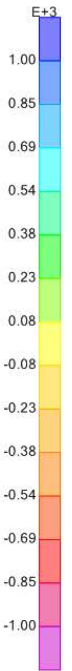
Initial and Long-Term Effective Tendon Stress (C-D)



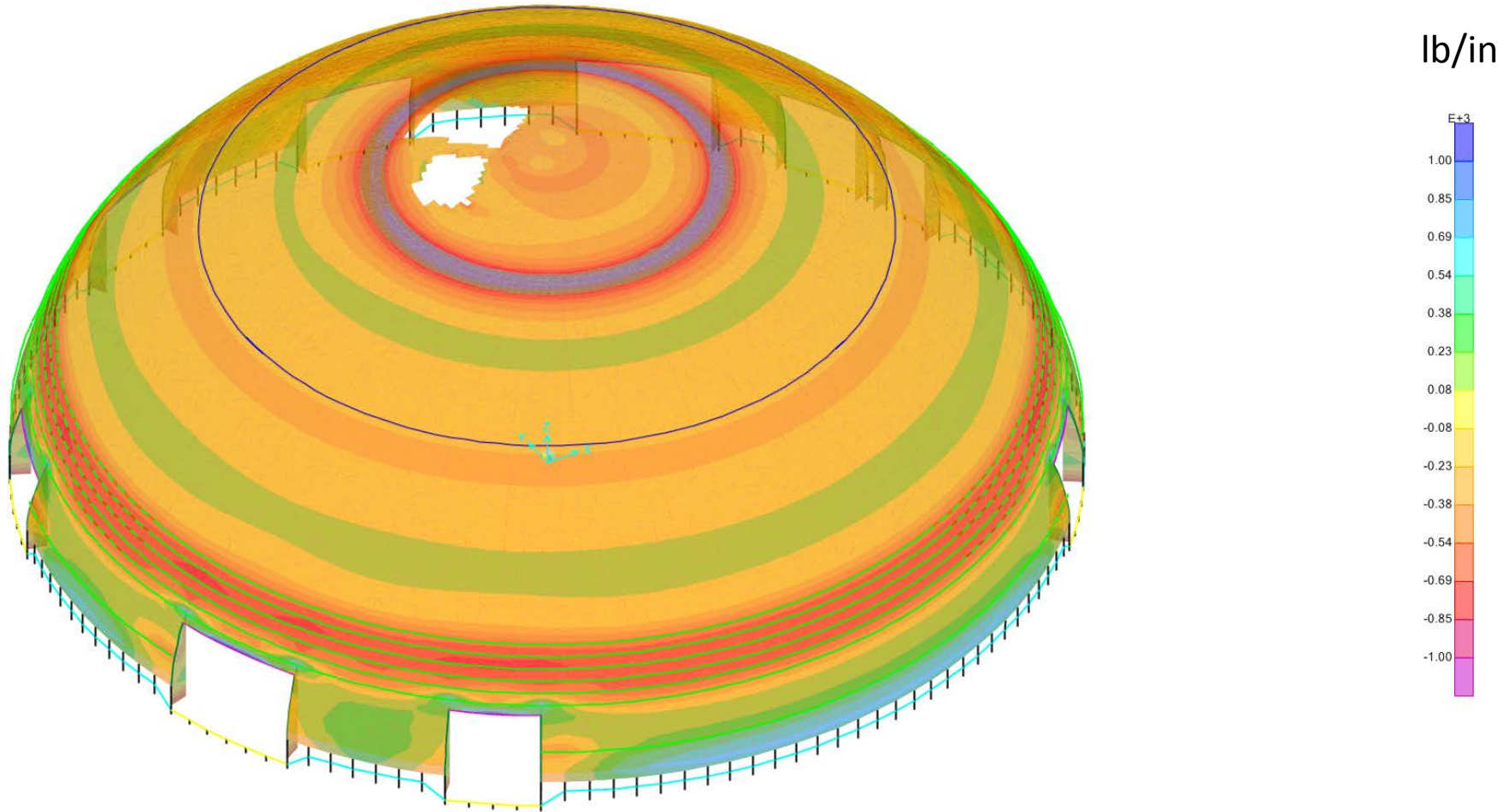
DL — Circumferential Force, F11 (+ = tension)



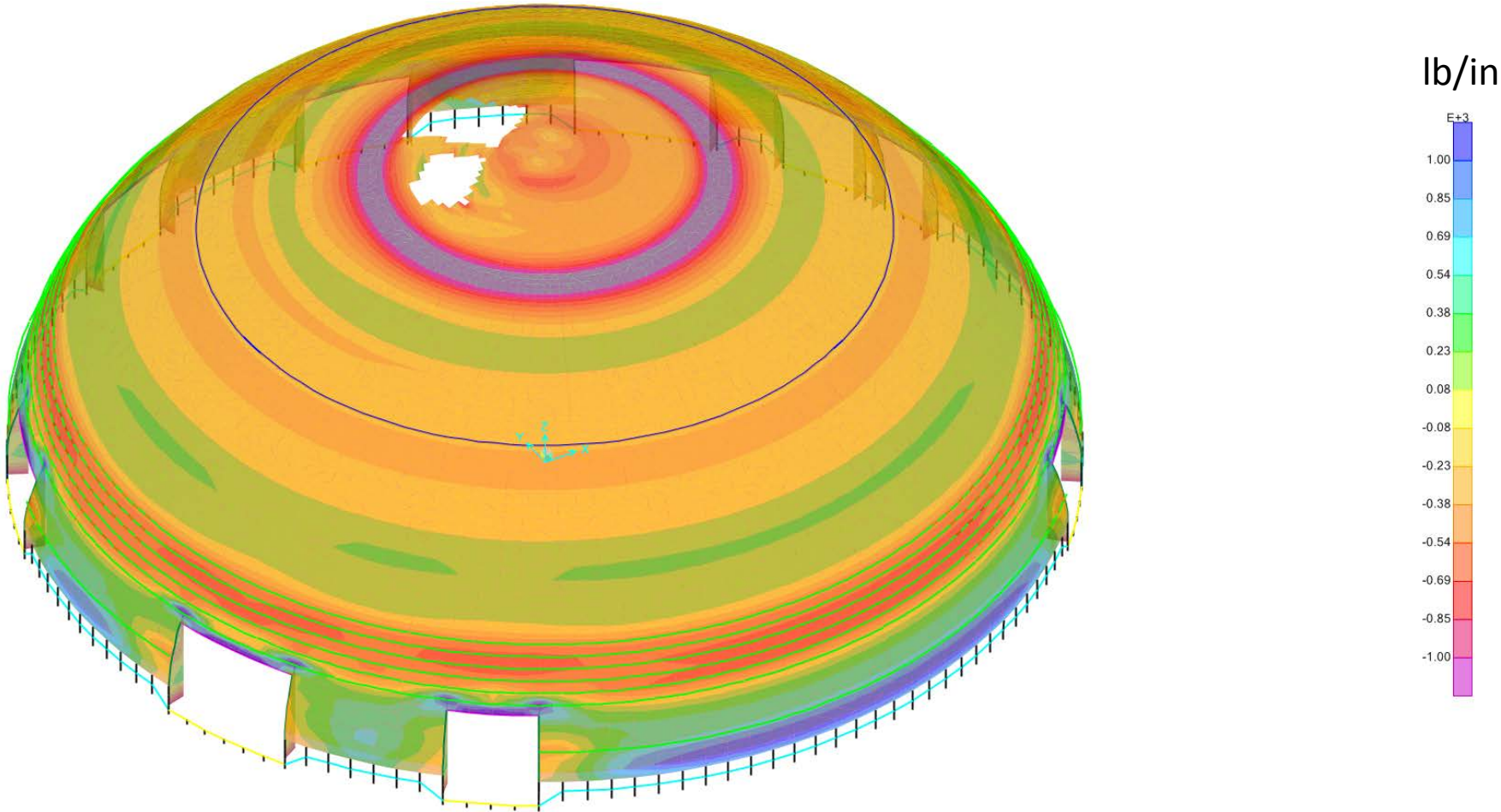
lb/in



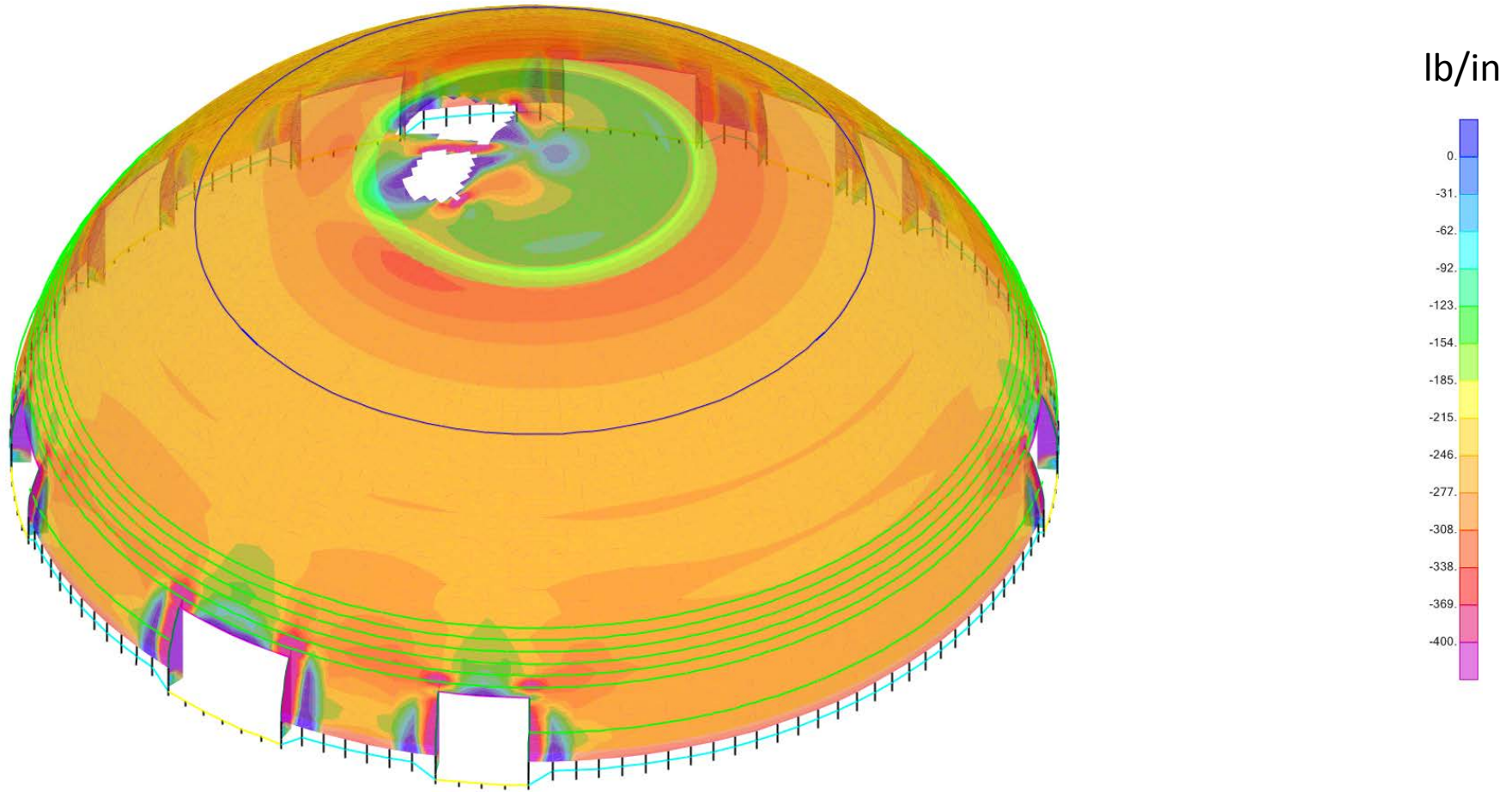
DL + PT — Circumferential Force, F11 (+ = tension)



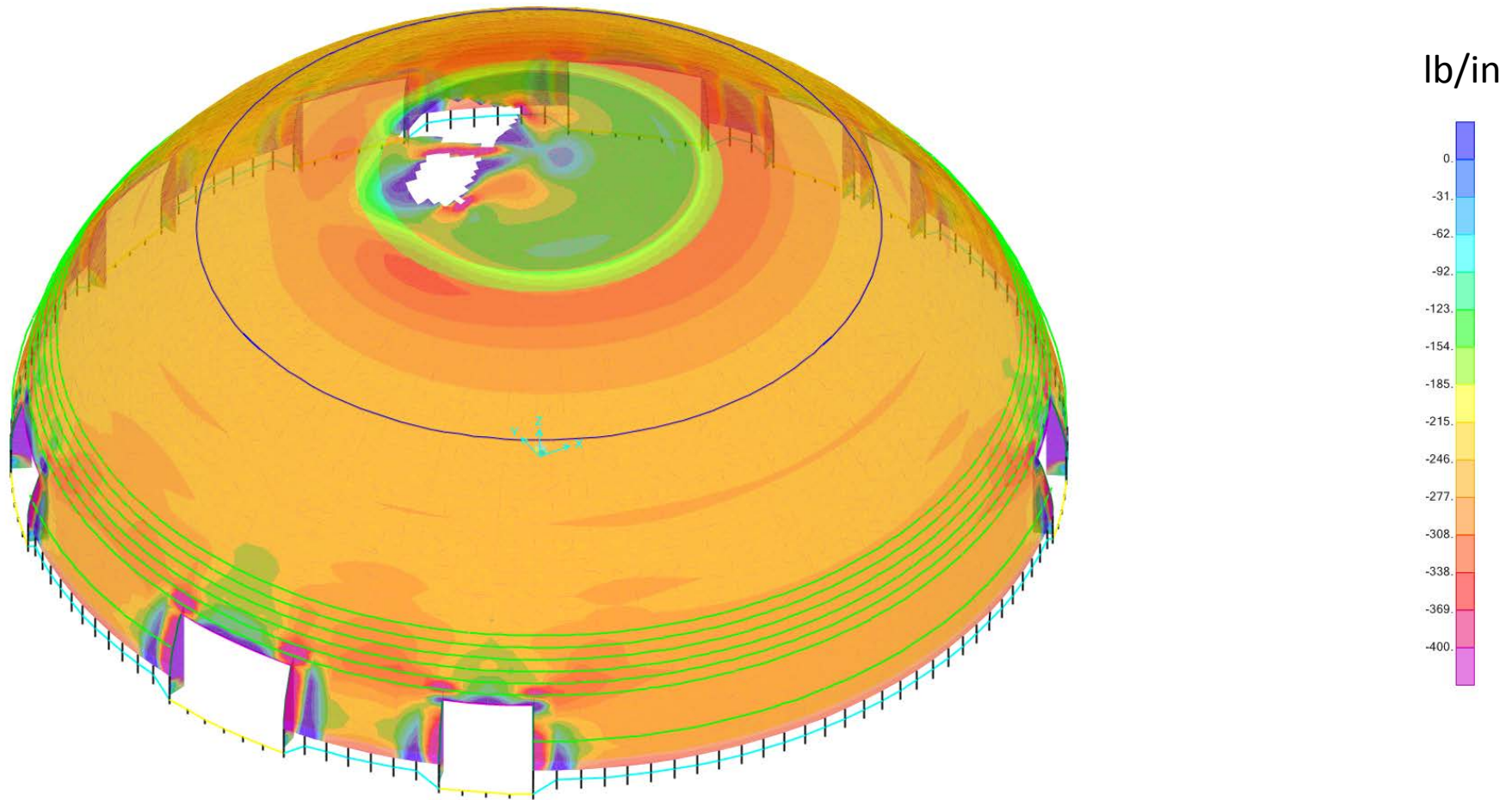
DL + PT + SL (Bal) — Circumferential Force,
F11 (+ = tension)



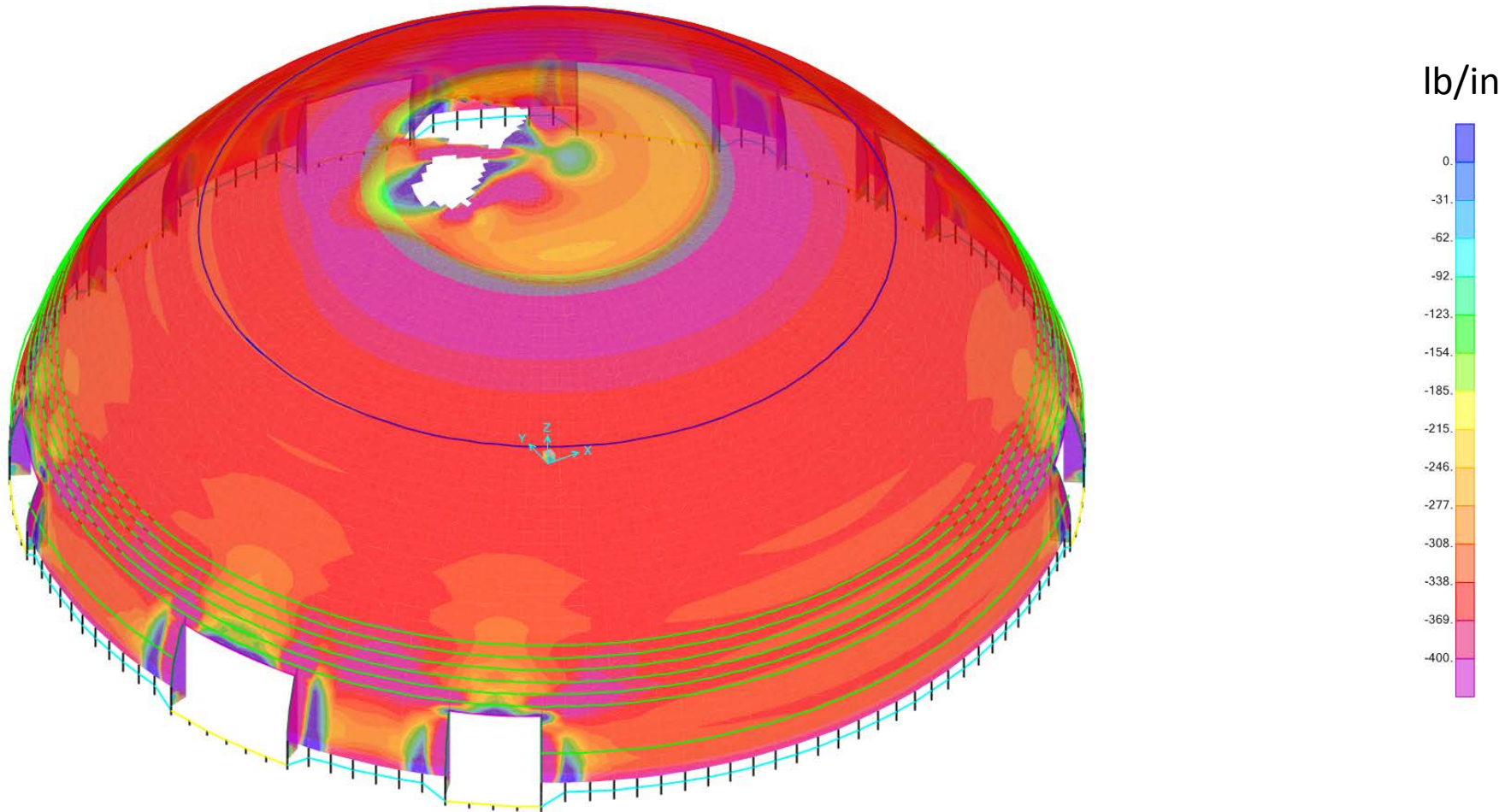
DL — Meridional Force, F22 (+ = tension)



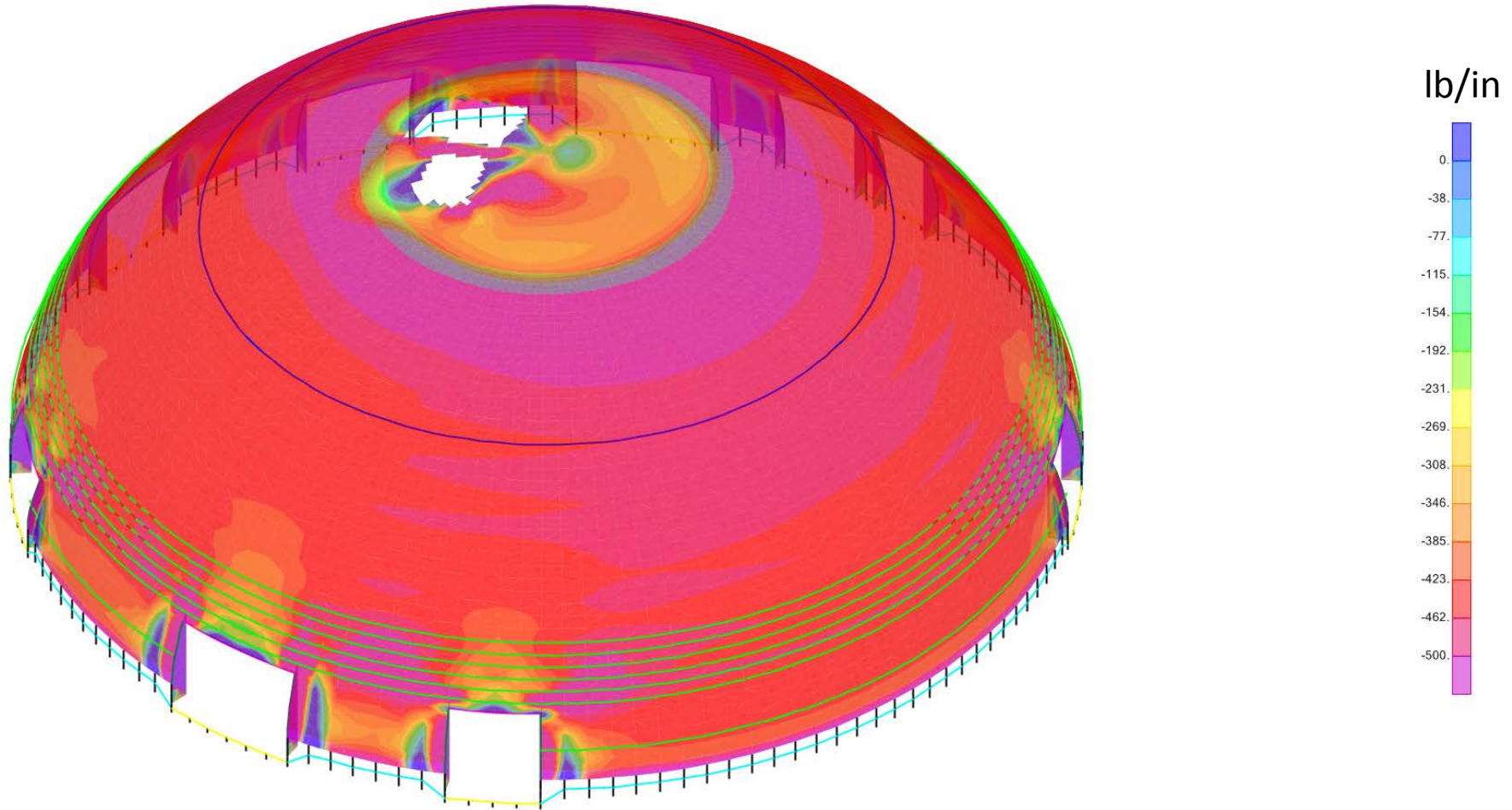
DL + PT — Meridional Force, F22 (+ = tension)

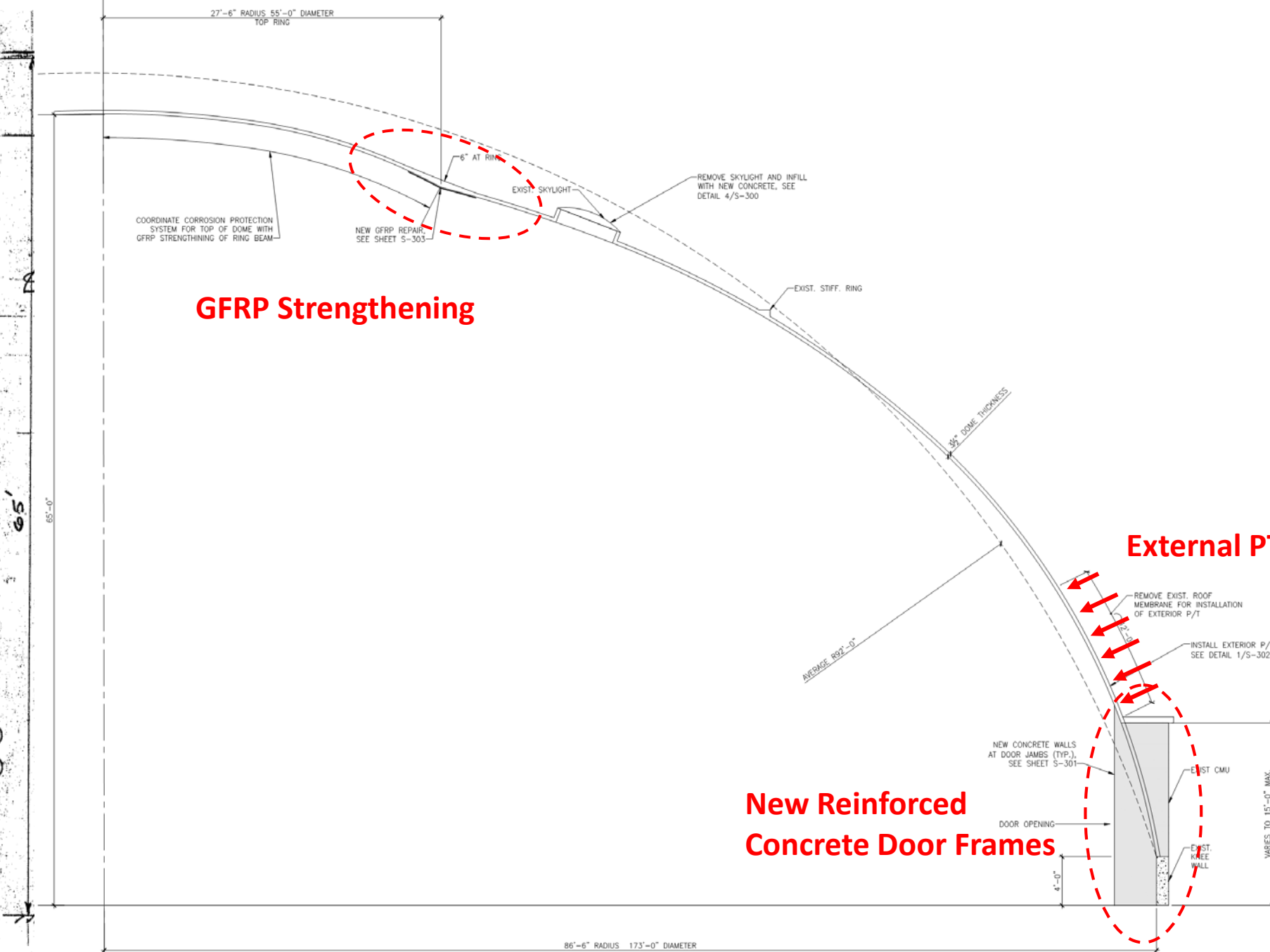


DL + PT + SL (Bal) — Meridional Force, F22 (+ = tension)



1.2DL+1.0PT+1.6SL(Bal)+0.5WL — Meridional Force, F22 (+ = tension)





GFRP Strengthening

External PT

New Reinforced Concrete Door Frames

COORDINATE CORROSION PROTECTION SYSTEM FOR TOP OF DOME WITH GFRP STRENGTHENING OF RING BEAM

NEW GFRP REPAIR, SEE SHEET S-303

6" AT RING

EXIST. SKYLIGHT

REMOVE SKYLIGHT AND INFILL WITH NEW CONCRETE, SEE DETAIL 4/S-300

EXIST. STIFF. RING

BY DOME THICKNESS

AVERAGE RPT - 0"

REMOVE EXIST. ROOF MEMBRANE FOR INSTALLATION OF EXTERIOR P/T

INSTALL EXTERIOR P/T, SEE DETAIL 1/S-302

NEW CONCRETE WALLS AT DOOR JAMBS (TYP.), SEE SHEET S-301

DOOR OPENING

EXIST. CMU

EXIST. WIRE WALL

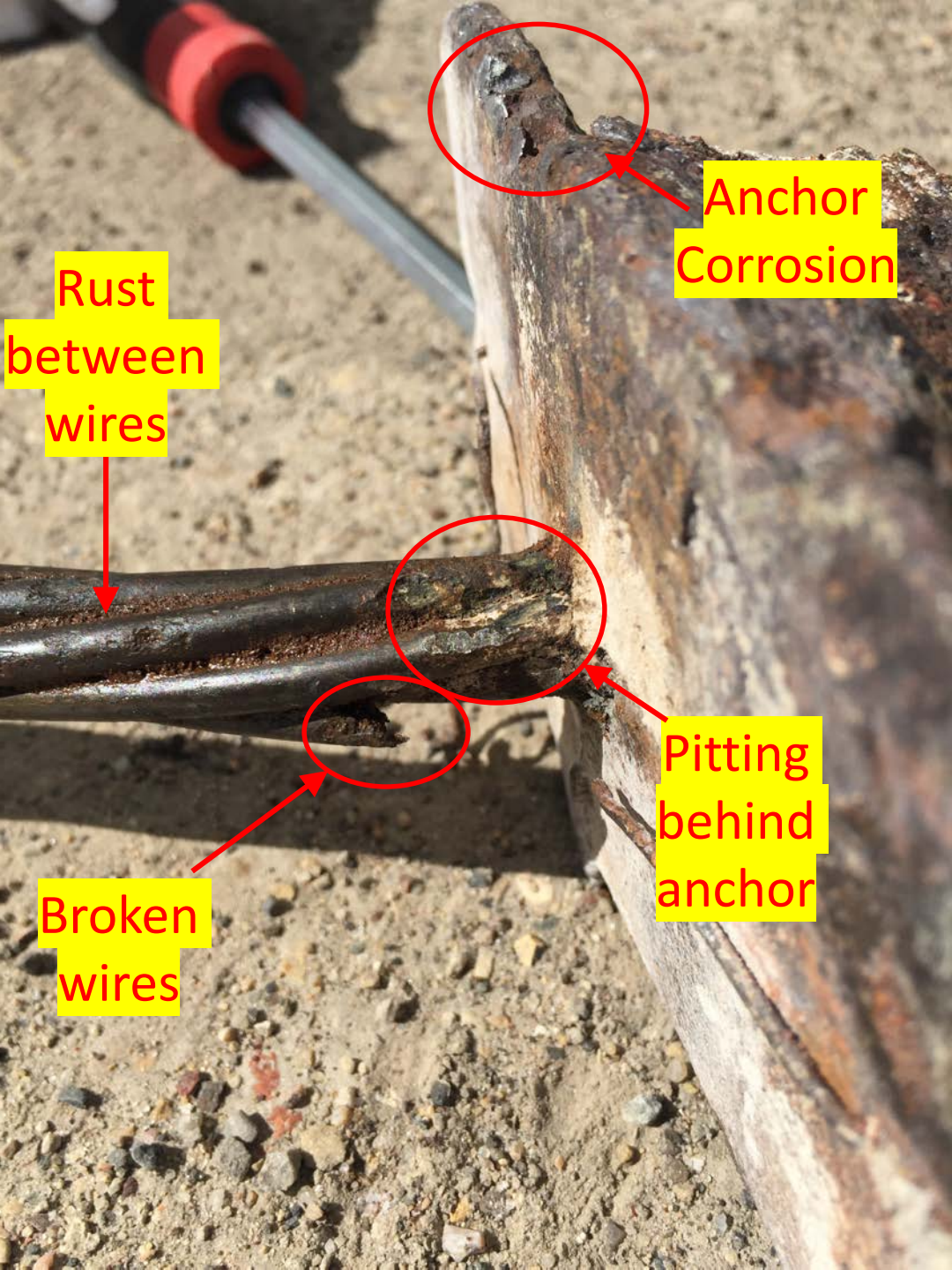
VARIES TO 15'-0" MAX.

65'

27'-6" RADIUS 55'-0" DIAMETER TOP RING

86'-6" RADIUS 173'-0" DIAMETER

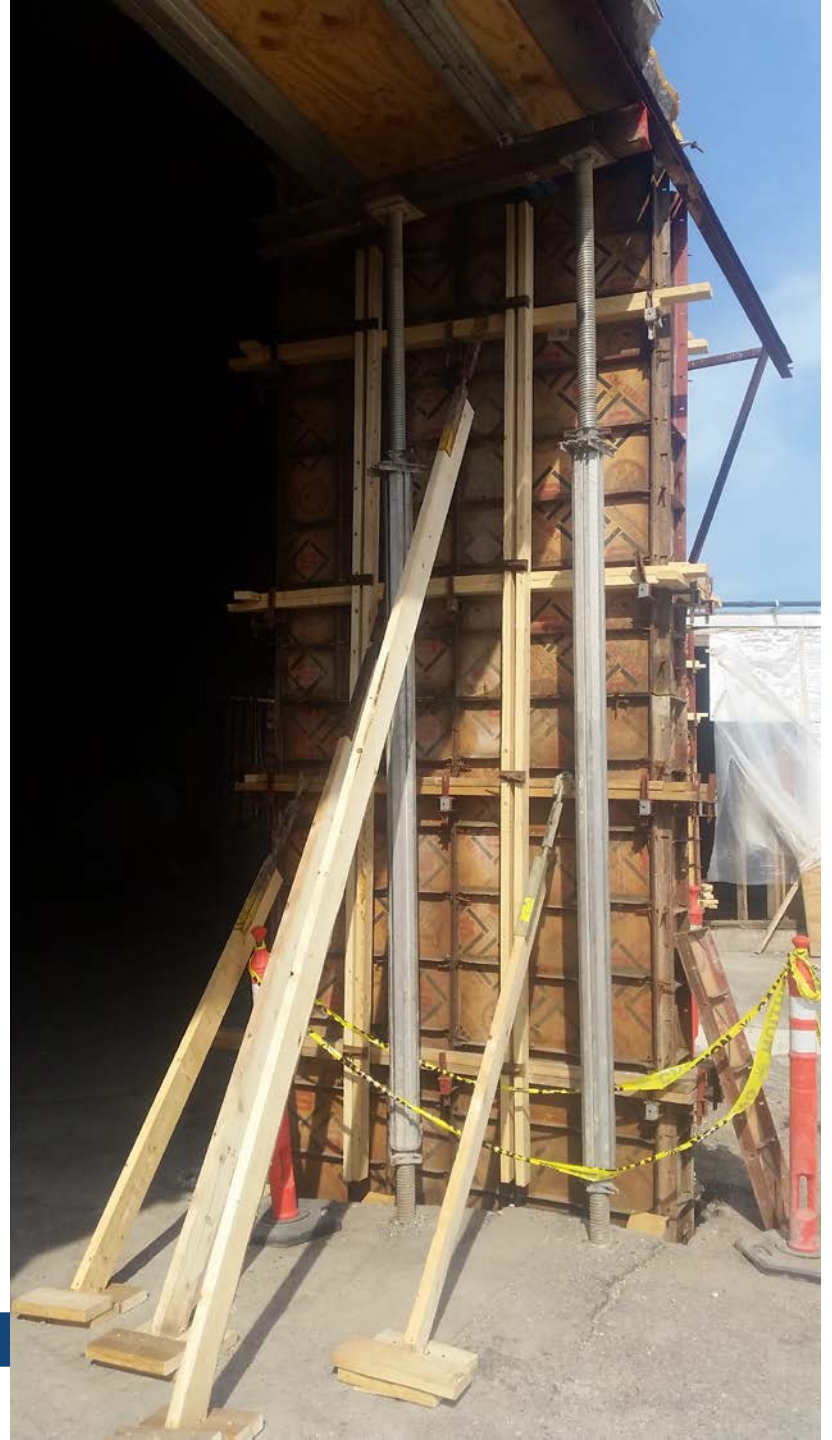














Infill Skylights



Epoxy Injection

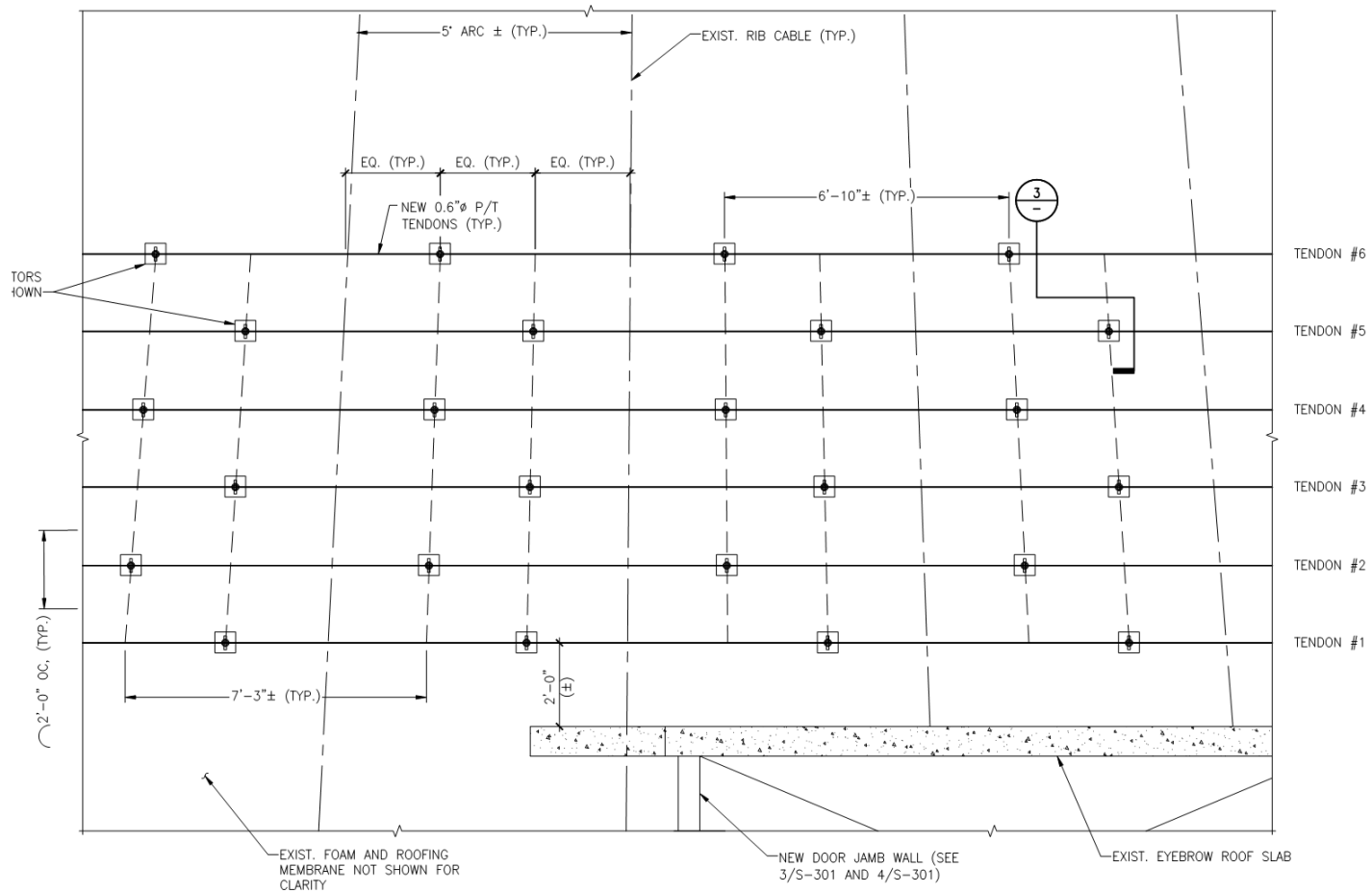


GFRP Strengthening



External PT





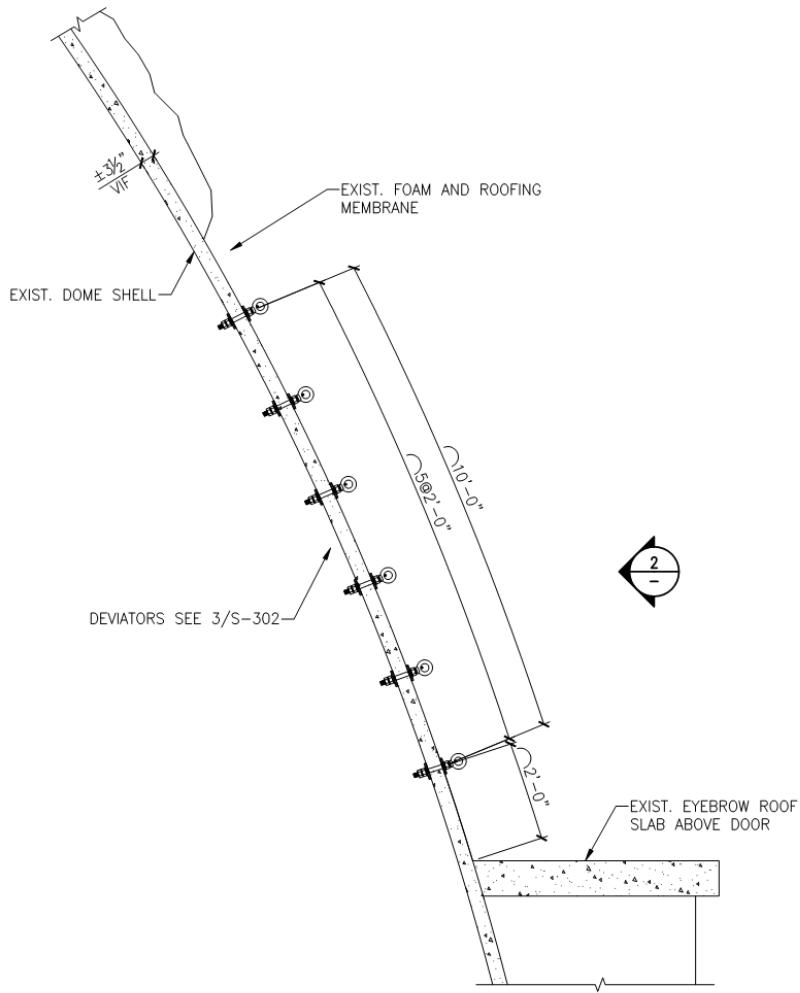
TENDON STRESSING PATTERN			
TENDON NUMBER	% OF MAXIMUM JACKING STRESS		
	STAGE 1	STAGE 2	STAGE 3
1	(1) 50%		(9) 100%
2		(6) 100%	
3	(2) 50%		(8) 100%
4		(5) 100%	
5	(3) 50%		(7) 100%
6		(4) 100%	

NOTES:

- (#) = ORDER OF TENDON STRESSING

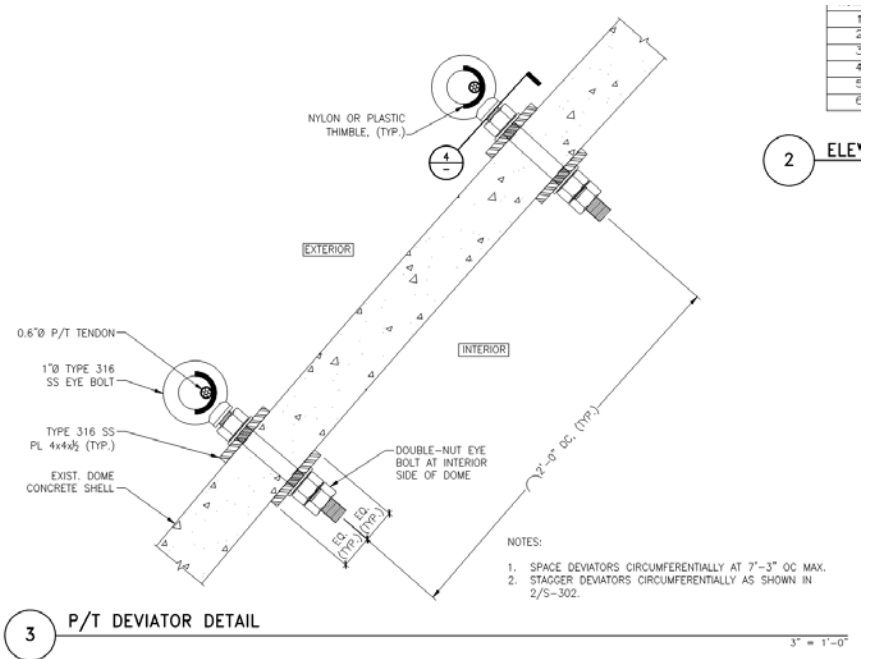
2 ELEVATION OF DEVIATOR LAYOUT

1/2" = 1'-0"



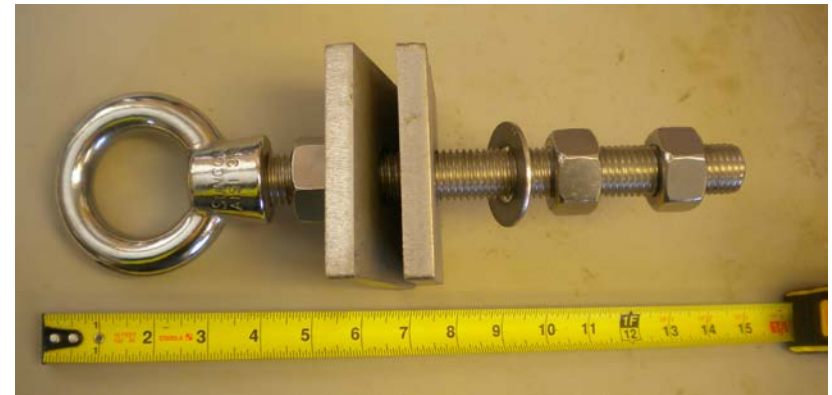
**1 SECTION AT DOME
(SHOWING NEW DEVIATORS)**

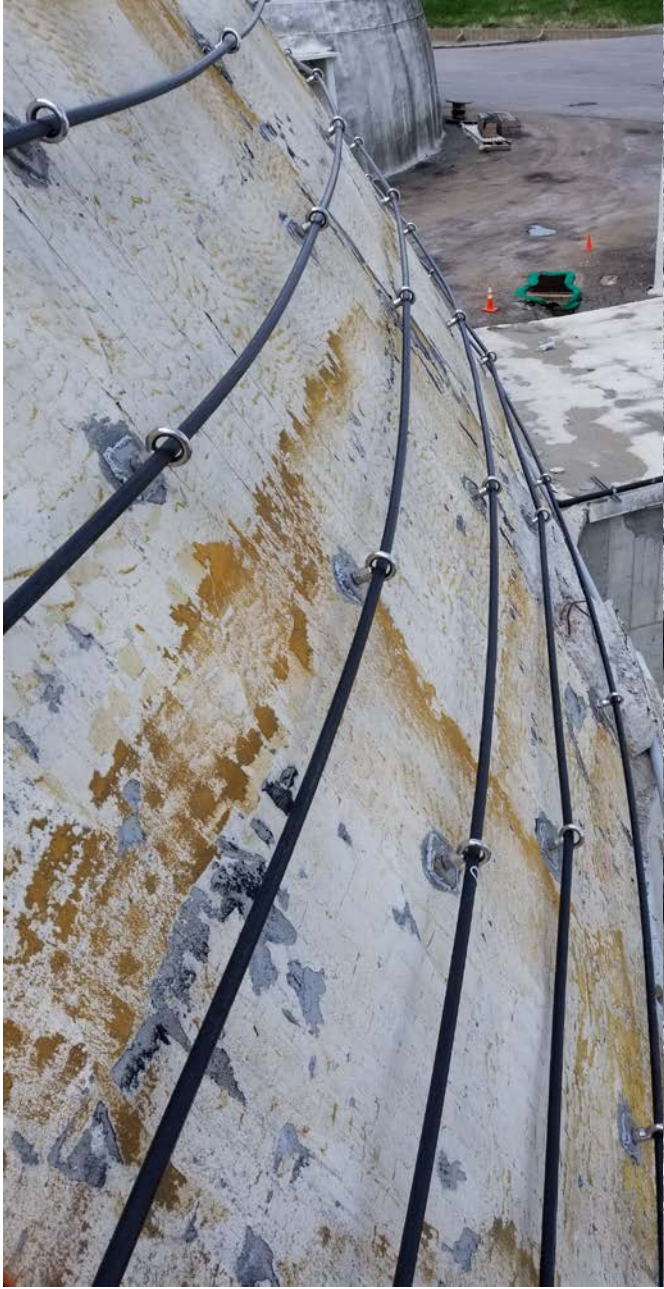
1/2" = 1'-0"



3 P/T DEVIATOR DETAIL

- NOTES:
1. SPACE DEVIATORS CIRCUMFERENTIALLY AT 7'-3" OC MAX.
 2. STAGGER DEVIATORS CIRCUMFERENTIALLY AS SHOWN IN 2/S-302.







JLG LIFT

United Rentals

1002011

E6





Keys to Success

- Teamwork
 - Owner
 - Production staff
 - Structural engineer staff
 - Construction team
 - Suppliers
- Evaluation to identify root cause of distress
- Proper structural analysis and design
- Constructability review during design phase
- Industrial safety mentality

Questions?

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